

Vertical dynamic interaction between suction caissons in tetrapod arrangements for offshore wind turbines

Jacob D.R. Bordón*, Fidel García, Luis A. Padrón, Juan J. Aznárez and Orlando Maeso

* Instituto Universitario de Sistemas Inteligentes y Aplicaciones Numéricas en Ingeniería
Universidad de Las Palmas de Gran Canaria
Campus Universitario de Tafira, Las Palmas de Gran Canaria 35017, Spain
e-mails: {jacobdavid.rodriquezbordon, fidel.garcia, juanjose.aznarez, orlando.maeso}@ulpgc.es

ABSTRACT

Offshore wind energy is playing an important role in the future mix of renewable sources for electrical power production. Although floating technology is taking off in a powerful way, more mature fixed wind turbines are still strongly growing in number and capabilities [1]. For challenging soft soils, suction caissons are often considered for geotechnical and ease of installation/removal reasons. In this contribution, the vertical dynamic interaction of a tetrapod arrangement of suction caissons is studied through an specially tailored boundary element - shell finite element coupled model. The model is an evolution of a previous one [2], where the boundary element hypersingular formulation required to deal with the shell coupling is no longer needed, and an already available multilayered viscoelastic half-space Green's function [3] can be hence used. The discretization is thus reduced to only the suction caisson skirt and lid. The dynamic interaction is observed from the point of view of stiffnesses. The influence of the foundation spacing and soil properties on the resulting impedance matrix is studied. As expected, the most relevant factor on the dynamic interaction is the spacing. Impedance curves contain local minima and maxima at frequencies corresponding to wavelength fractions of the spacing.

ACKNOWLEDGMENTS

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- [1] EWEA (The European Wind Energy Association). Offshore Wind in Europe: Key trends and statistics 2020. Wind Europe, 2021.
- [2] J.D.R. Bordón, J.J. Aznárez, O. Maeso. Dynamic model of open shell structures buried in poroelastic soils. *Computational Mechanics* 60, 269–288, 2017.
- [3] R.Y.S. Pak, B.B. Guzina. Three-dimensional Green's functions for a multilayered half-space in displacement potentials. *Journal of Engineering Mechanics* 128, 449–461, 2002.

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Congress on Numerical Methods in Engineering
CMN 2022

Las Palmas de Gran Canaria, Spain, 13 September, 2022

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Introduction

Methodology

Results and discussion

Conclusions

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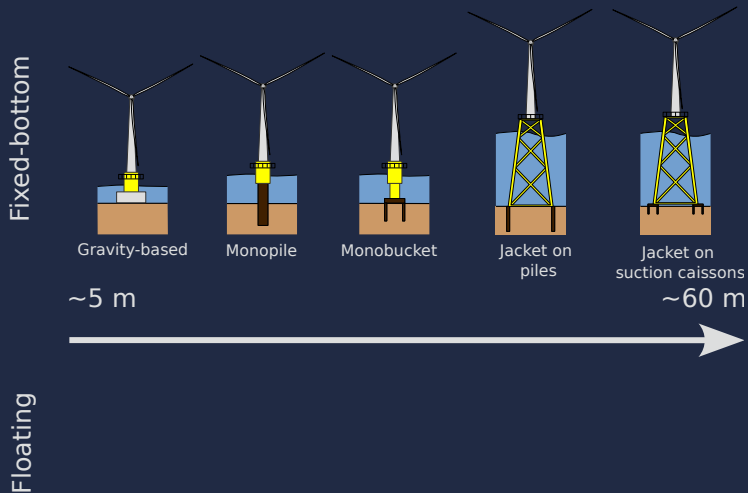
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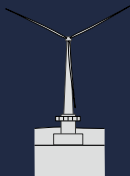
Conclusions

Offshore Wind Turbines (OWT)

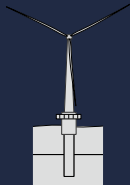


Offshore Wind Turbines (OWT)

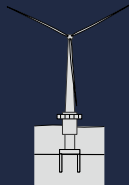
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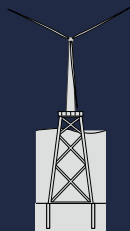
Gravity-based



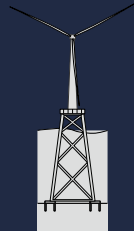
Monopile



Monobucket

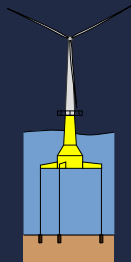


Jacket on piles

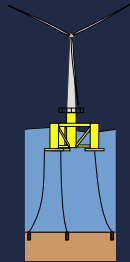


Jacket on suction caissons

Floating



Tension-Leg Platform (TLP)



Semi-submersible

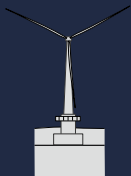


Spar

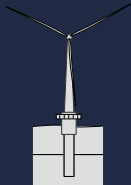
~60 m
to
~300 m

Offshore Wind Turbines (OWT)

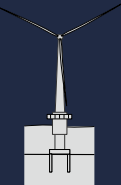
Fixed-bottom



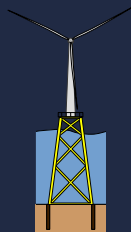
Gravity-based



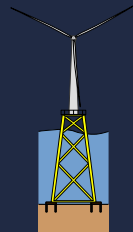
Monopile



Monobucket



Jacket on
piles



Jacket on
suction caissons

Floating



OWT environment and design

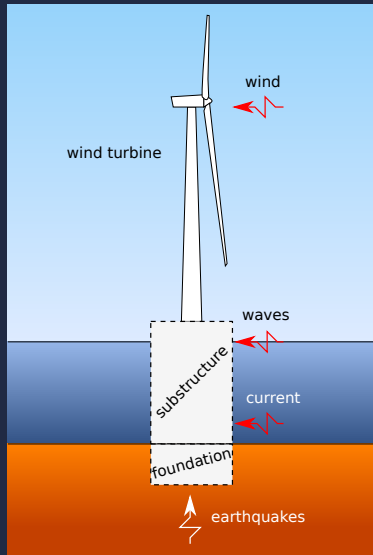


Figure: Challenging environment and design. Image source: D. Schroeder

Fundamental frequency challenge

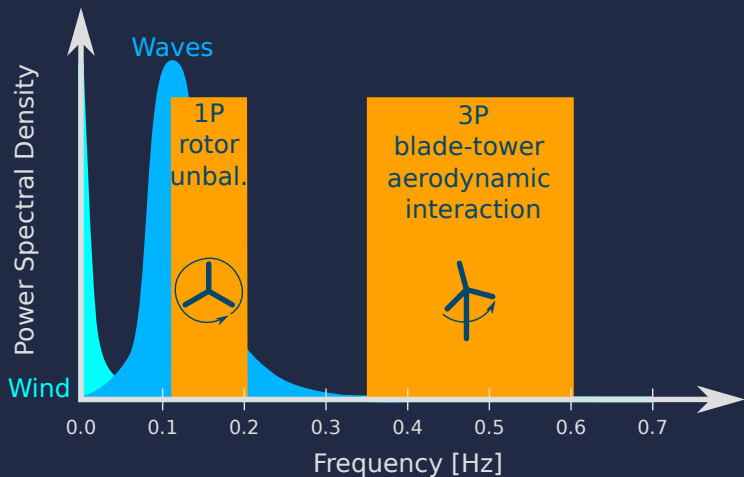


Figure: Dynamic loads and OWT fundamental frequency

Fundamental frequency challenge

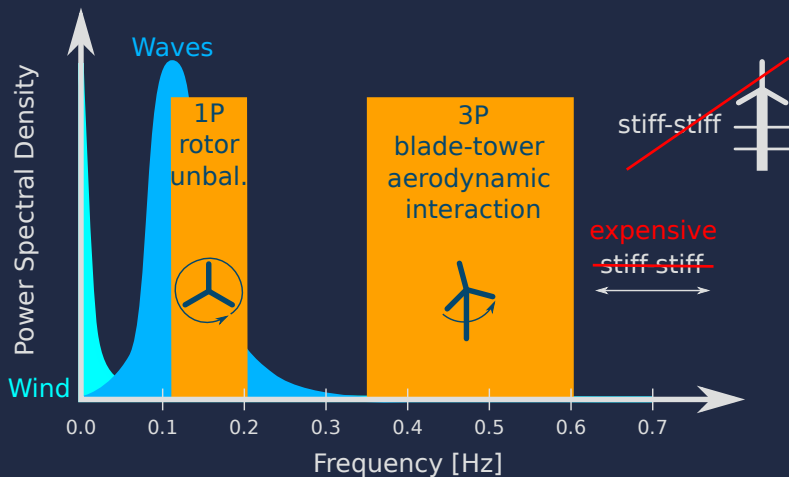


Figure: Dynamic loads and OWT fundamental frequency

Fundamental frequency challenge

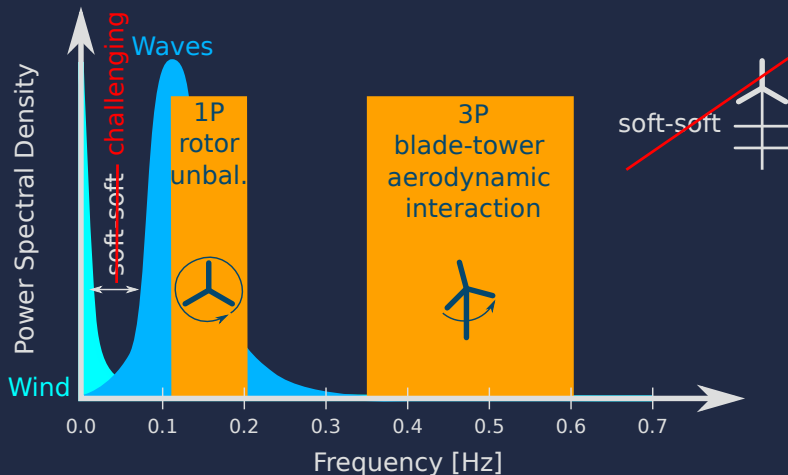


Figure: Dynamic loads and OWT fundamental frequency

Fundamental frequency challenge

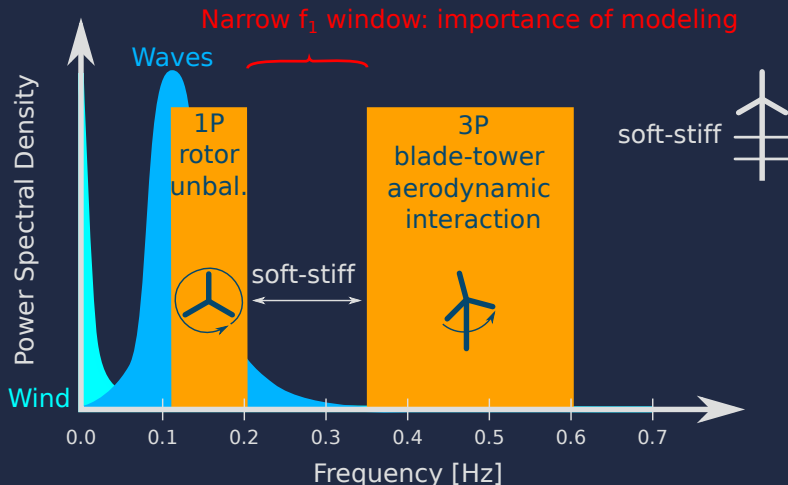
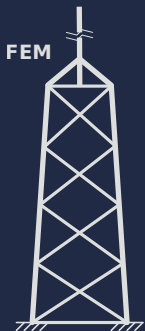


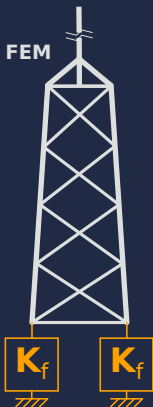
Figure: Dynamic loads and OWT fundamental frequency

OWT Soil-Structure Interaction (SSI) modeling



Rigid base

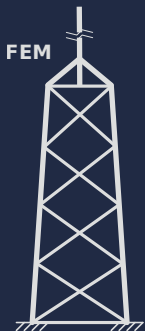
No SSI



Flexible base
without foundation
interaction

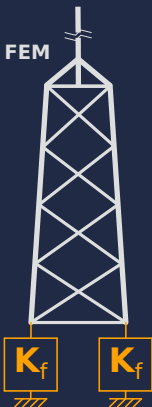
SSI

OWT Soil-Structure Interaction (SSI) modeling

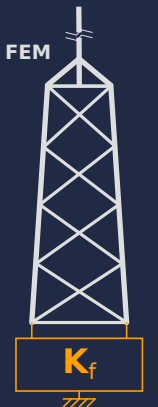


Rigid base

No SSI



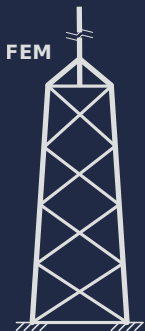
Flexible base
without foundation
interaction



Flexible base
with foundation
interaction

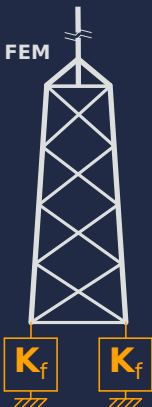
SSI

OWT Soil-Structure Interaction (SSI) modeling

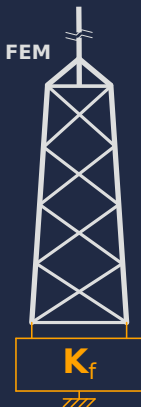


Rigid base

No SSI

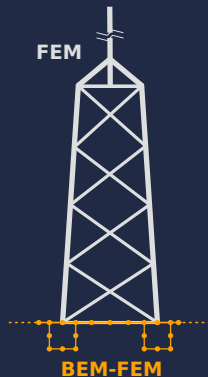


Flexible base
without foundation
interaction



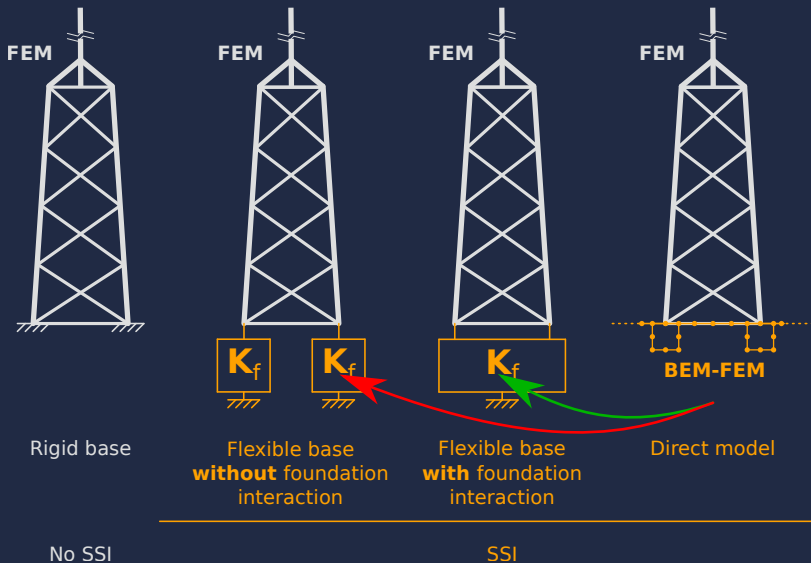
Flexible base
with foundation
interaction

SSI

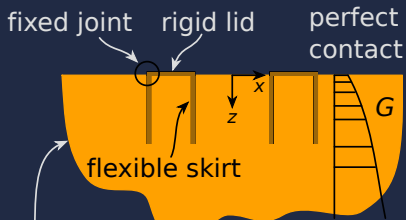
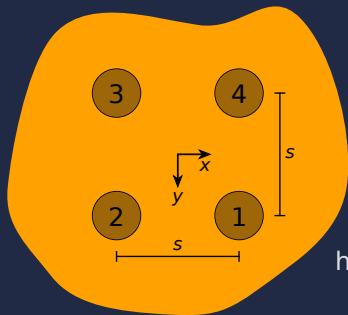
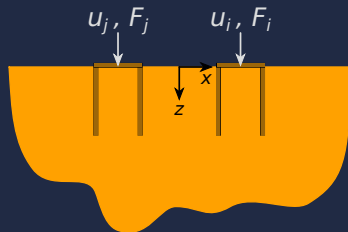
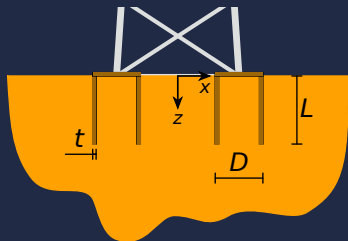


Direct model

OWT Soil-Structure Interaction (SSI) modeling



Modeling assumptions and aim



horizontally layered
half-space

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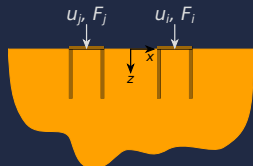
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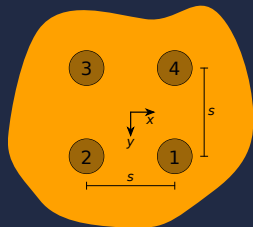
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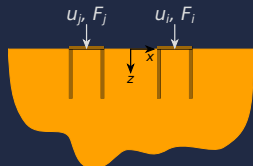
Vertical interaction



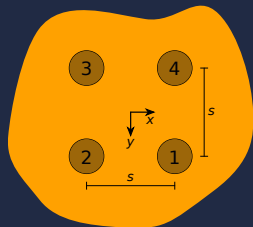
$$\begin{Bmatrix} F_1 \\ F_2 \\ F_3 \\ F_4 \end{Bmatrix} = \begin{bmatrix} K_{11} & K_{12} & K_{13} & K_{14} \\ & K_{22} & K_{23} & K_{24} \\ & & K_{33} & K_{34} \\ \text{sym.} & & & K_{44} \end{bmatrix} \begin{Bmatrix} u_1 \\ u_2 \\ u_3 \\ u_4 \end{Bmatrix}$$



Vertical interaction

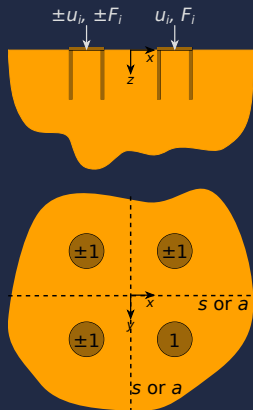


$$\begin{Bmatrix} F_1 \\ F_2 \\ F_3 \\ F_4 \end{Bmatrix} = \begin{bmatrix} K_{11} & K_{12} & K_{13} & K_{12} \\ & K_{11} & K_{12} & K_{13} \\ & & K_{11} & K_{12} \\ \text{sym.} & & & K_{11} \end{bmatrix} \begin{Bmatrix} u_1 \\ u_2 \\ u_3 \\ u_4 \end{Bmatrix}$$



K_{11} self
 K_{12} side
 K_{13} diagonal

Vertical interaction



Exploiting the one-quarter symmetry:

- ▶ Plane zx : symmetry or antisymmetry
- ▶ Plane yz : symmetry or antisymmetry

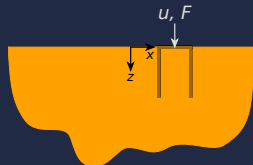
Superposition of ss , sa , as ($\equiv sa$) and aa :

$$K_{11} = \frac{1}{4} (K_{ss} + 2K_{sa} + K_{aa})$$

$$K_{12} = \frac{1}{4} (K_{ss} - K_{aa})$$

$$K_{13} = \frac{1}{4} (K_{ss} - 2K_{sa} + K_{aa})$$

Vertical interaction

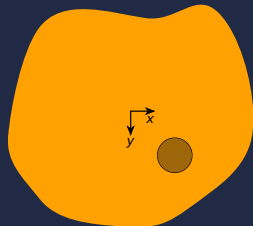


As $s \rightarrow \infty$:

$$K_{11} \rightarrow K_{\text{isolated}}$$

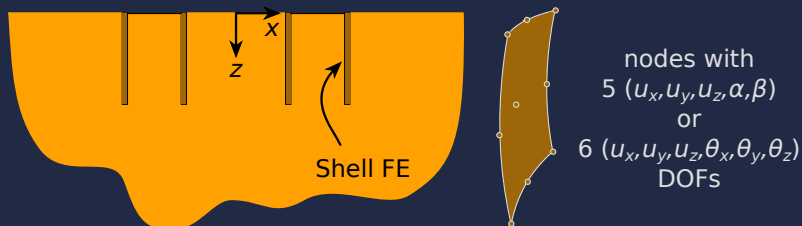
$$K_{12} \rightarrow 0$$

$$K_{13} \rightarrow 0$$



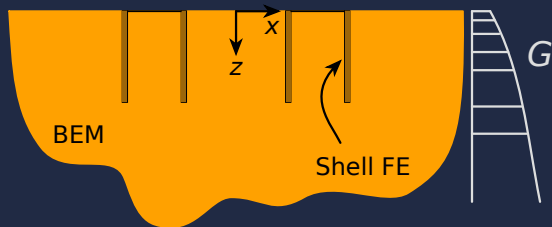
where K_{isolated} is the impedance of an isolated foundation.

BEM-FEM numerical approach (overview)



- ▶ Skirts: Shell Finite Elements.
 - ▶ Shell degenerated from solid - MITC9¹

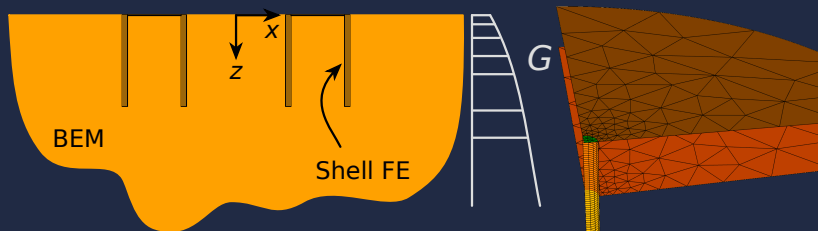
BEM-FEM numerical approach (overview)



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- ▶ Soil: Boundary Element Method.

¹Bucalem & Bathe. Higher-order MITC general shell elements. Int J Num Meth Eng, 36, 1993.

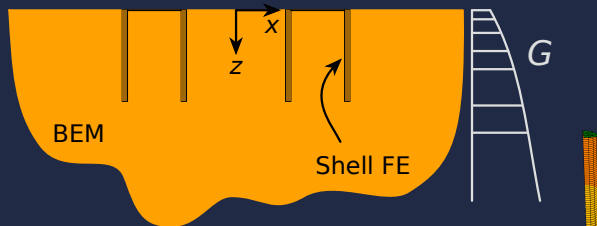
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BEM-FEM numerical approach (overview)



- ▶ Skirts: Shell Finite Elements.
 - ▶ Shell degenerated from solid - MITC9¹
- ▶ Soil: Boundary Element Method.
 - ▶ Green's function for horizontally layered half-space²

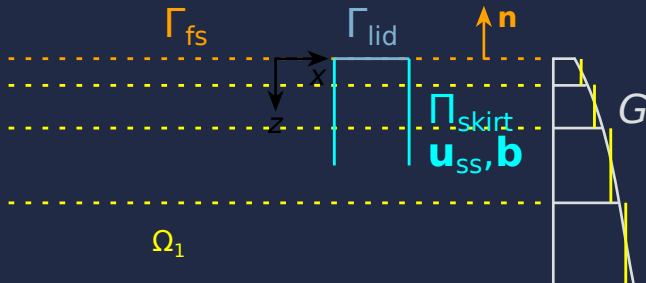
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²Pak & Guzina. Three-dimensional Green's functions for a multilayered half-space in displacement potentials. J Eng Mech, 128, 2002.

BEM-FEM numerical approach (some details)

Displacement Boundary Integral Equation:

$$c_{lk}^i u_k^i + \oint_{\Gamma_{\text{lid}}} t_{lk}^* u_k \, d\Gamma = \int_{\Gamma_{\text{lid}}} u_{lk}^* t_k \, d\Gamma + \int_{\Pi_{\text{skirt}}} u_{lk}^* b_k \, d\Pi \Rightarrow \mathbf{H}\mathbf{u} = \mathbf{G}\mathbf{t} + \mathbf{G}_b\mathbf{b}$$



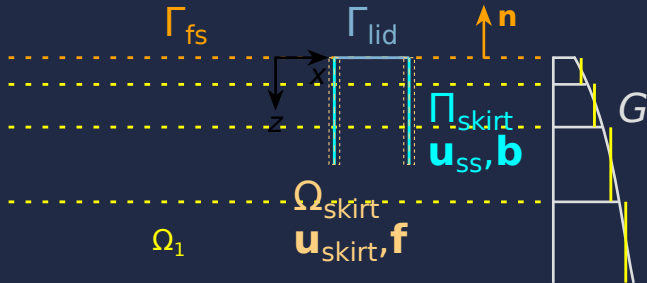
BEM-FEM numerical approach (some details)

Displacement Boundary Integral Equation:

$$c_{lk}^i u_k^i + \oint_{\Gamma_{\text{lid}}} t_{lk}^* u_k \, d\Gamma = \int_{\Gamma_{\text{lid}}} u_{lk}^* t_k \, d\Gamma + \int_{\Omega_{\text{skirt}}} u_{lk}^* b_k \, d\Omega \Rightarrow \mathbf{H}\mathbf{u} = \mathbf{G}\mathbf{t} + \mathbf{G}_b\mathbf{b}$$

Shell finite element equilibrium equation (element level):

$$\Omega_{\text{skirt}} : \mathbf{K}^{(e)} \mathbf{a}^{(e)} - \mathbf{Q}^{(e)} \mathbf{f}^{(e)} = \mathbf{q}^{(e)}$$



BEM-FEM numerical approach (some details)

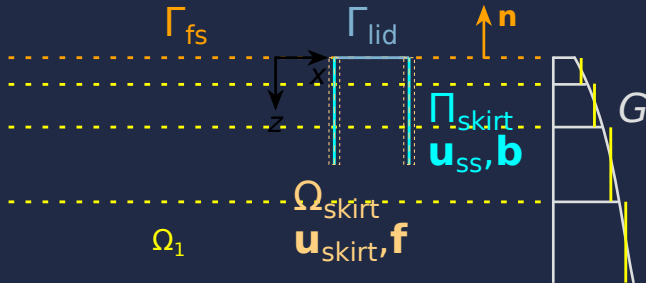
Displacement Boundary Integral Equation:

$$c_{lk}^i u_k^i + \oint_{\Gamma_{\text{lid}}} t_{lk}^* u_k \, d\Gamma = \int_{\Gamma_{\text{lid}}} u_{lk}^* t_k \, d\Gamma + \int_{\Pi_{\text{skirt}}} u_{lk}^* b_k \, d\Pi \Rightarrow \mathbf{H}\mathbf{u} = \mathbf{G}\mathbf{t} + \mathbf{G}_b\mathbf{b}$$

Shell finite element equilibrium equation (element level):

$$\Omega_{\text{skirt}} : \mathbf{K}^{(e)} \mathbf{a}^{(e)} - \mathbf{Q}^{(e)} \mathbf{f}^{(e)} = \mathbf{q}^{(e)}$$

Perfect contact: compatibility $\mathbf{u}_{\text{ss}} = \mathbf{u}_{\text{skirt}}$ and equilibrium $\mathbf{b} + \mathbf{f} = \mathbf{0}$.



Horizontally Layered Half-space Green's function

- Regularization of the integral:

$$\oint_{\Gamma_{\text{lid}}} t_{lk}^* u_k \, d\Gamma = \int_{\Gamma_{\text{lid}}} [t_{lk}^* - (t_{lk}^*)_{\text{Mindlin}}] u_k \, d\Gamma + \oint_{\Gamma_{\text{lid}}} (t_{lk}^*)_{\text{Mindlin}} u_k \, d\Gamma$$

where $(t_{lk}^*)_{\text{Mindlin}}$ are tractions of the homogeneous half-space GF in statics.

Horizontally Layered Half-space Green's function

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where $(t_{lk}^*)_{\text{Mindlin}}$ are tractions of the homogeneous half-space GF in statics.

- ▶ Overall computational cost – a trade-off between:
 - ▶ DOF reduction: smaller LSE
 - ▶ HLGF cost \gg fundamental solution cost: costly LSE

Horizontally Layered Half-space Green's function

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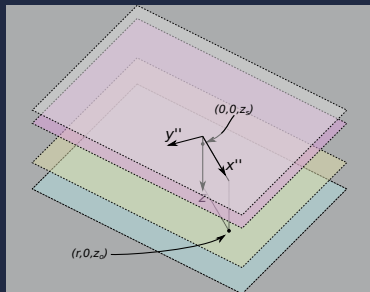
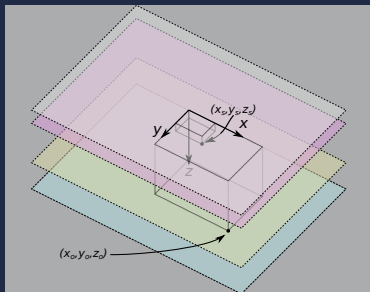
where $(t_{lk}^*)_{\text{Mindlin}}$ are tractions of the homogeneous half-space GF in statics.

- ▶ Overall computational cost – a trade-off between:
 - ▶ DOF reduction: smaller LSE
 - ▶ HLGF cost \gg fundamental solution cost: costly LSE
- ▶ Develop strategies to reduce the number of HLGF evaluations.

Horizontally Layered Half-space Green's function

a Build a database of HLGF evaluations $(u_{lk}^*, \sigma_{lkj}^*)$ based on:

- Axial symmetry: database indexed by (z_s, z_o, r)



b Mesh regularity

BEM-FEM model

Quadratic 9-node elements

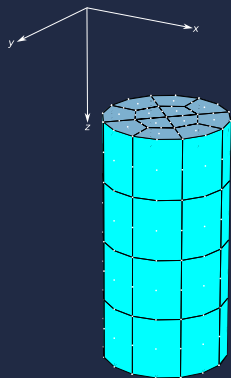
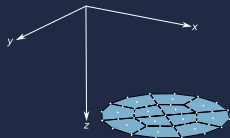


Figure: Example mesh: $L/D = 2$, $s/D = 4$

BEM-FEM model: lid

Quadratic 9-node boundary elements



BC:

$$\mathbf{u} = (0, 0, 1)$$

When solved:

$$K^{(lid)} = \int_{\Gamma_{lid}} t_z \, d\Gamma$$

Figure: Example mesh: $L/D = 2$, $s/D = 4$

BEM-FEM model: skirt

Quadratic 9-node shell finite elements (MITC9)

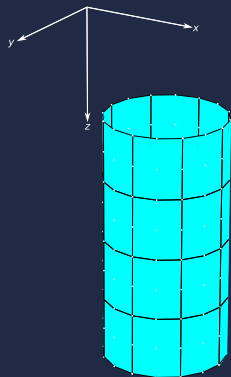


Figure: Example mesh: $L/D = 2$, $s/D = 4$

BC (top nodes):

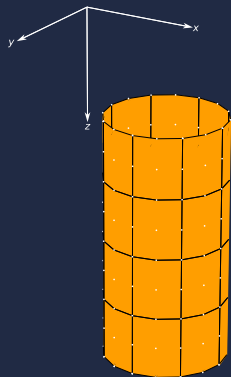
$$\begin{Bmatrix} u_x \\ u_y \\ u_z \\ \theta_x \\ \theta_y \\ \theta_z \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \\ 1 \\ 0 \\ 0 \\ 0 \end{Bmatrix}$$

When solved:

$$K^{(skirt)} = \sum_{\text{top nodes}} F_z^{(i)}$$

BEM-FEM model: soil-skirt interface

Quadratic 9-node BEM body load surface elements



Perfect contact conditions:

$$\mathbf{u}_{ss} = \mathbf{u}_{skirt}$$

$$\mathbf{b}_{ss} + \mathbf{f}_{skirt} = \mathbf{0}$$

Figure: Example mesh: $L/D = 2$, $s/D = 4$

Outline

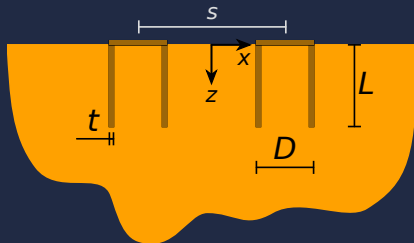
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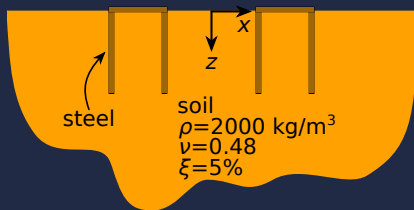
Conclusions

Cases: geometry

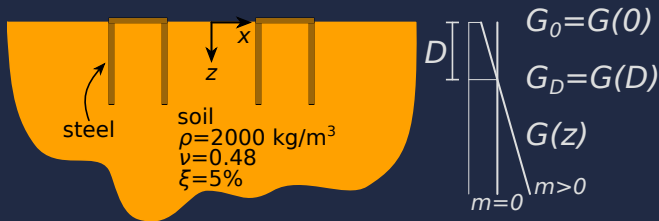


- ▶ Skirt thickness to diameter ratio: $\frac{t}{D} = 0.02$
- ▶ Depth to diameter ratios: $\frac{L}{D} = \{1, 2, 4\}$
- ▶ Spacing to diameter ratios: $\frac{s}{D} = \{2, 4, 8\}$

Cases: material properties



Cases: material properties

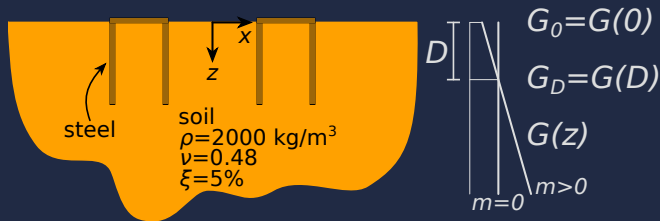


► Gibson soil: $G(z) = G_0 \left(1 + m \frac{z}{D} \right)$

► $m = \frac{G_D}{G_0} - 1 = \{0, 0.2, 0.4\}$

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Cases: material properties



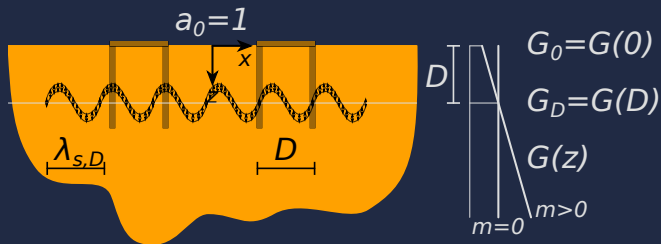
- ▶ Gibson soil: $G(z) = G_0 \left(1 + m \frac{z}{D} \right)$
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 - ▶ $\frac{G_D}{G_0} = \{1, 1.2, 1.4\}$
- ▶ Skirt-soil stiffness ratio: $J = \frac{G_{st}}{G_D} \frac{t}{D} = \{10, 100, 1000\}$

Cases: frequencies



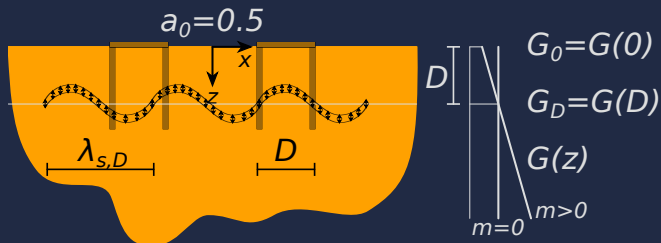
► Dimensionless frequency: $a_0 = \frac{D}{\lambda_{s,D}} = \frac{f D}{c_{s,D}}$

Cases: frequencies



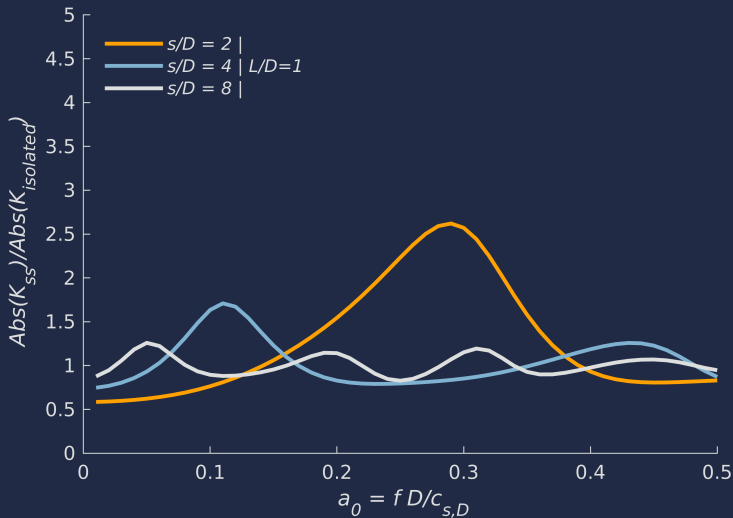
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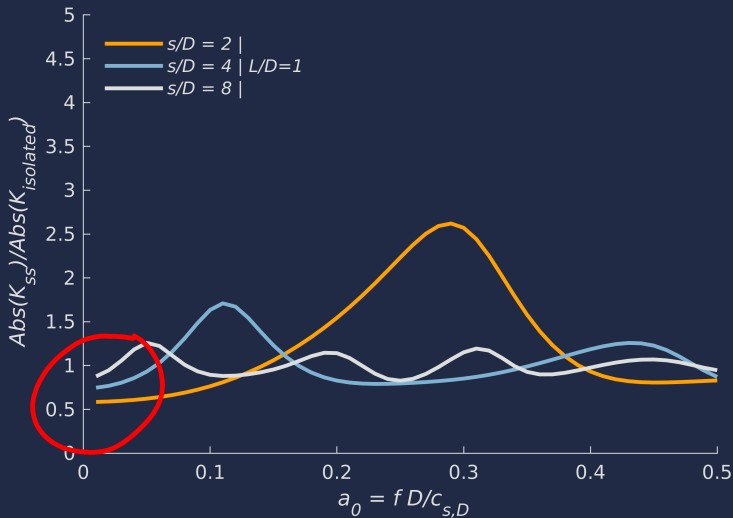


- Dimensionless frequency: $a_0 = \frac{D}{\lambda_{s,D}} = \frac{f D}{c_{s,D}} = (0, 0.5]$

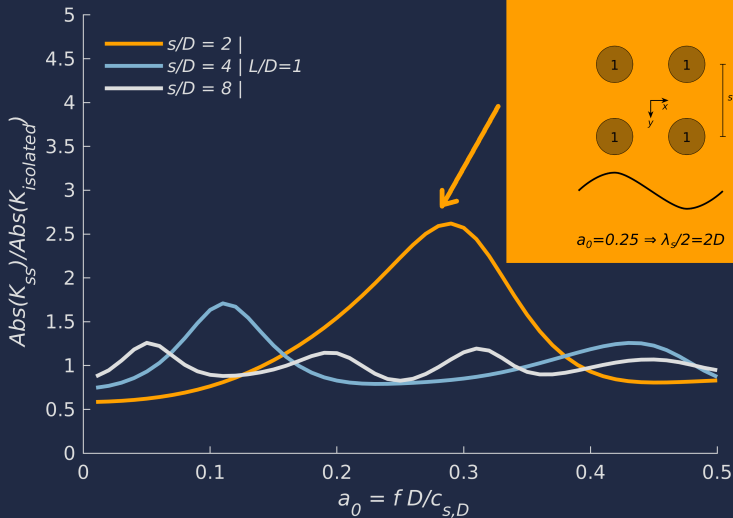
K_{SS} vs L/D vs s/D , $J = 100$, homogeneous soil



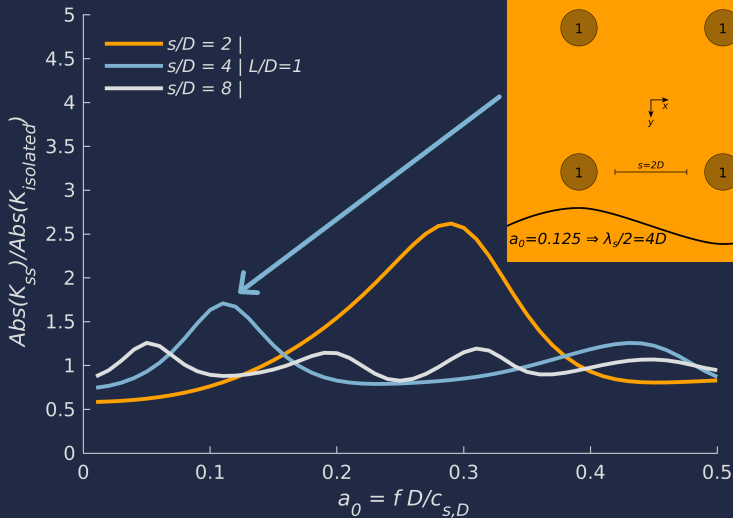
K_{SS} vs L/D vs s/D , $J = 100$, homogeneous soil



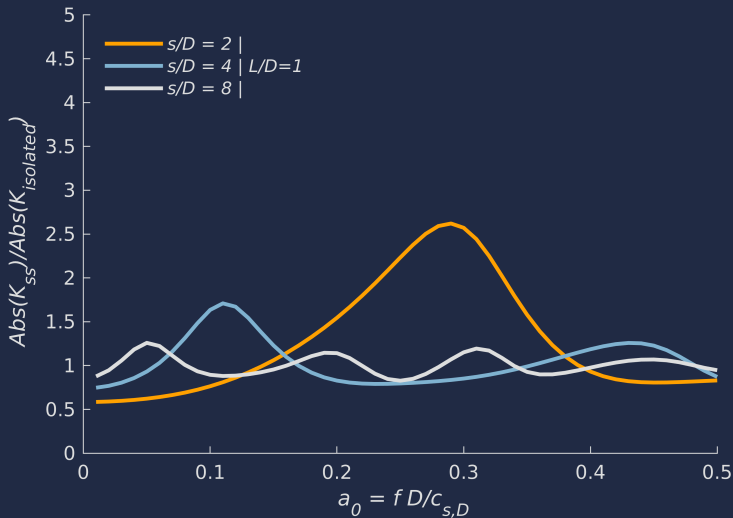
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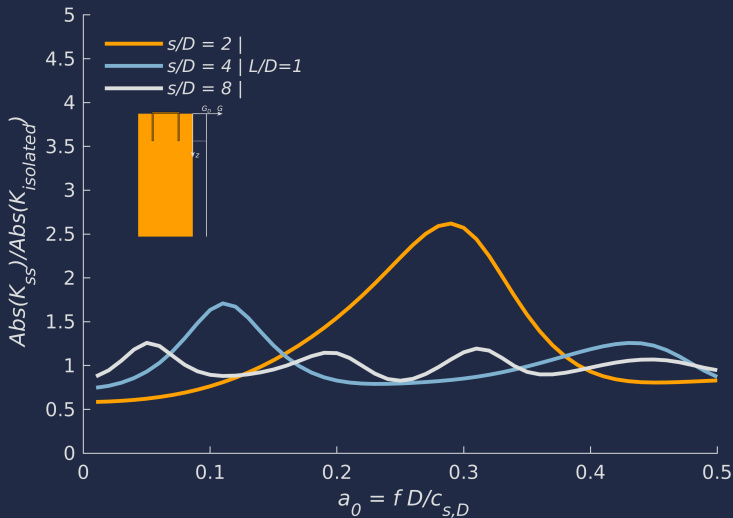
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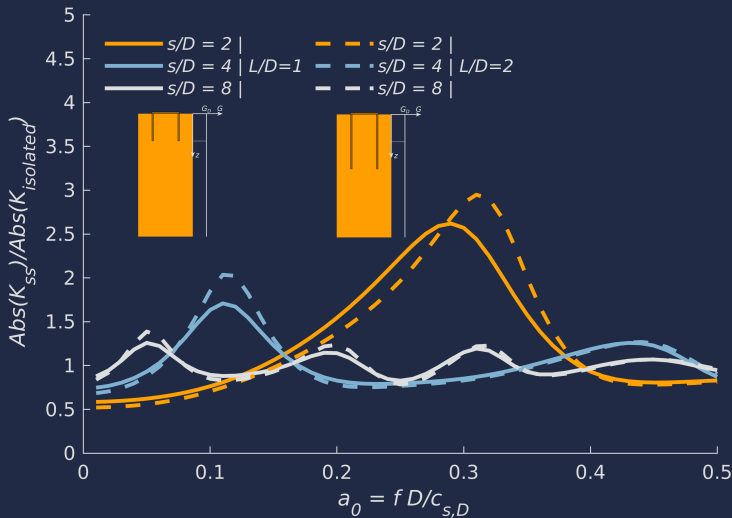
K_{SS} vs L/D vs s/D , $J = 100$, homogeneous soil



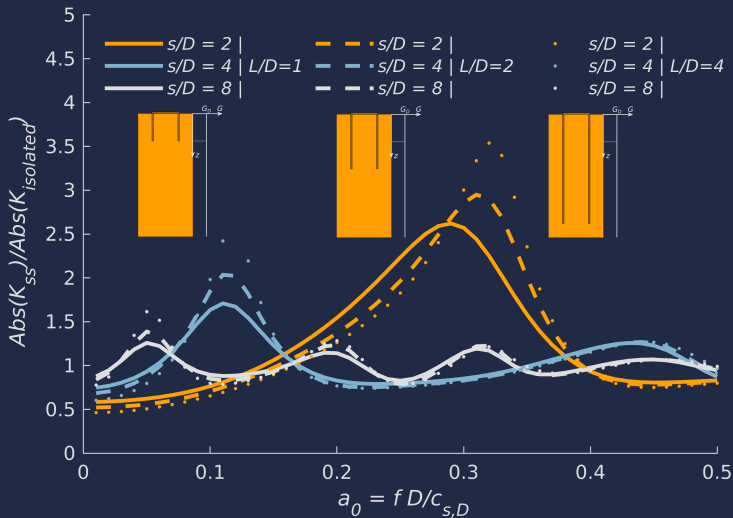
K_{SS} vs L/D vs s/D , $J = 100$, homogeneous soil



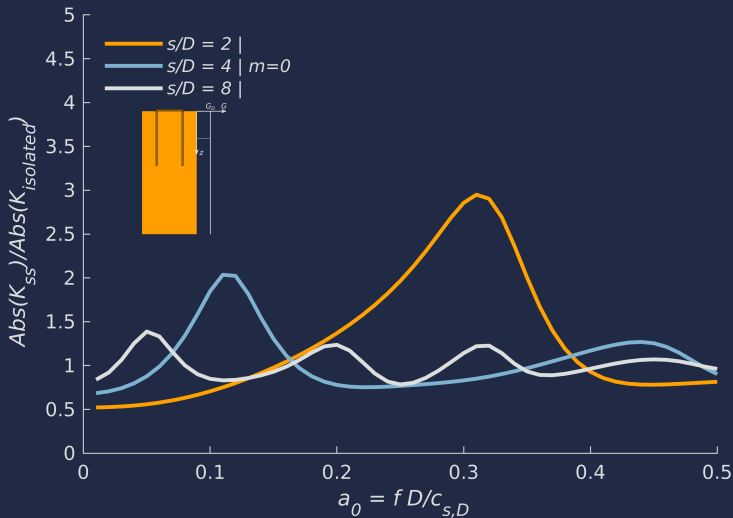
K_{SS} vs L/D vs s/D , $J = 100$, homogeneous soil



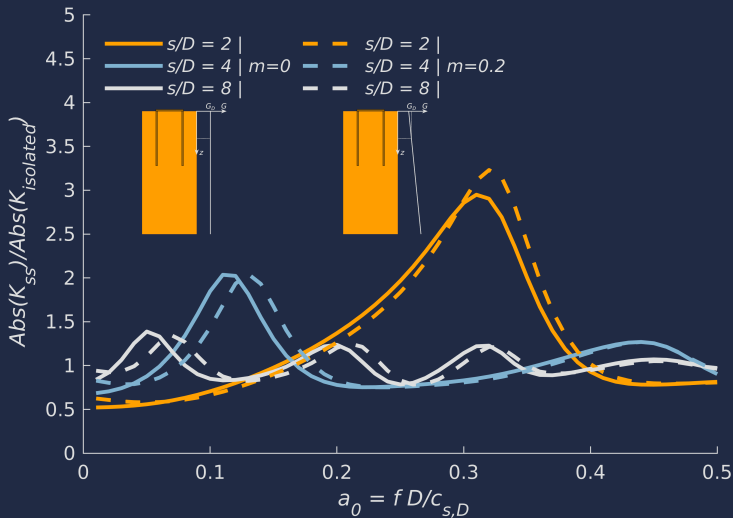
K_{SS} vs L/D vs s/D , $J = 100$, homogeneous soil



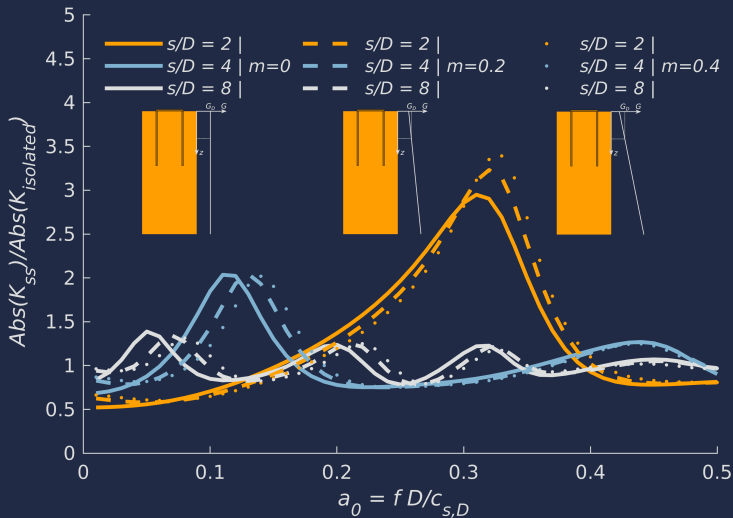
K_{SS} vs s/D vs m , $J = 100$, $L/D = 2$



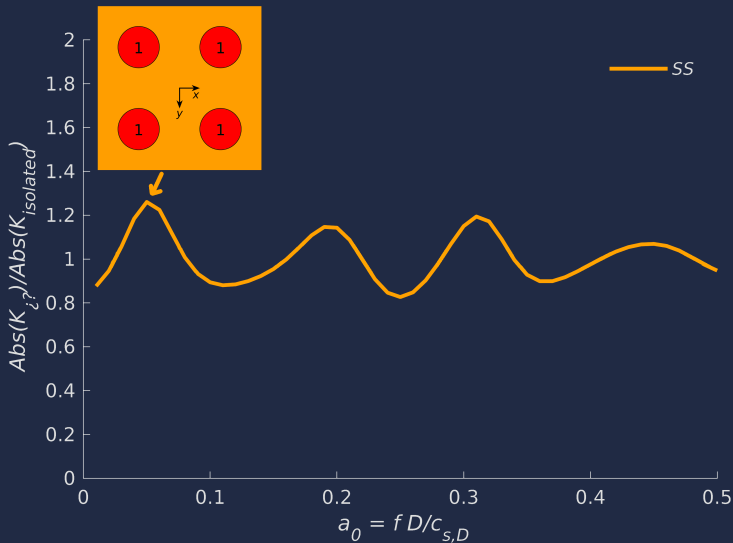
K_{SS} vs s/D vs m , $J = 100$, $L/D = 2$



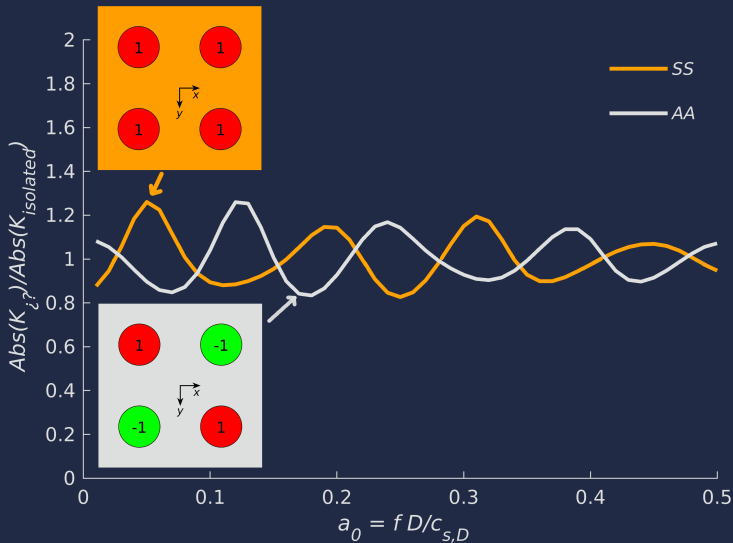
K_{SS} vs s/D vs m , $J = 100$, $L/D = 2$



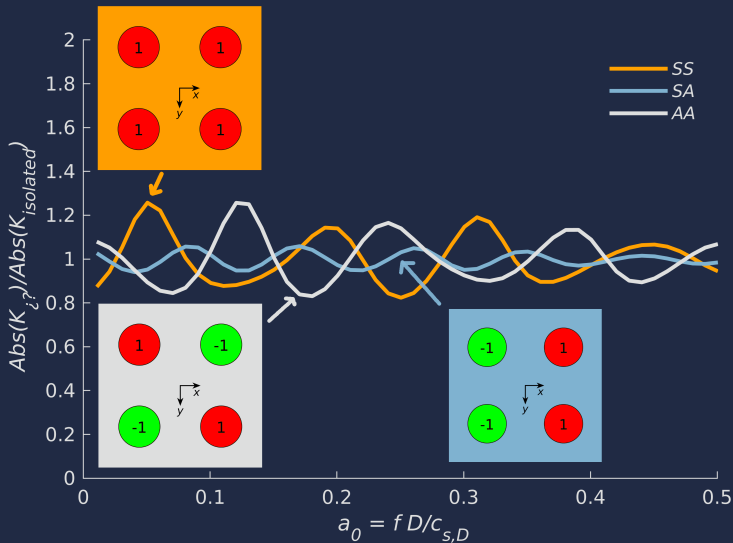
K_{aa} vs K_{sa} vs K_{ss} , $s/D = 8$, $L/D = 1$, homog.



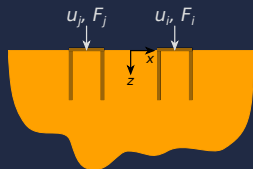
K_{aa} vs K_{sa} vs K_{ss} , $s/D = 8$, $L/D = 1$, homog.



K_{aa} vs K_{sa} vs K_{ss} , $s/D = 8$, $L/D = 1$, homog.

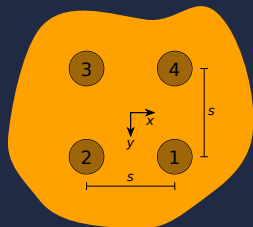


Vertical interaction (K_{11} , K_{12} , K_{13})



$$\begin{Bmatrix} F_1 \\ F_2 \\ F_3 \\ F_4 \end{Bmatrix} = \begin{bmatrix} K_{11} & K_{12} & K_{13} & K_{12} \\ & K_{11} & K_{12} & K_{13} \\ & & K_{11} & K_{12} \\ \text{sym.} & & & K_{11} \end{bmatrix} \begin{Bmatrix} u_1 \\ u_2 \\ u_3 \\ u_4 \end{Bmatrix}$$

Superposition of ss, sa, as (\equiv sa) and aa:

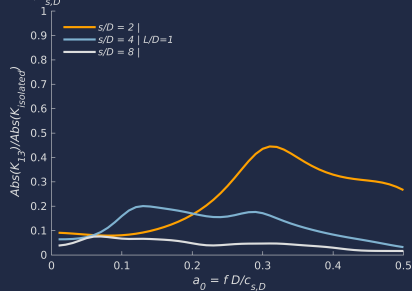
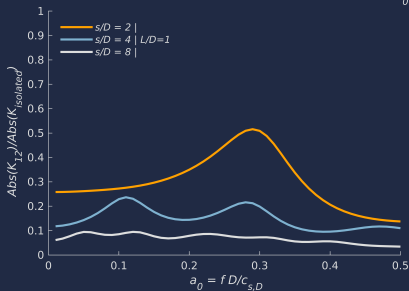
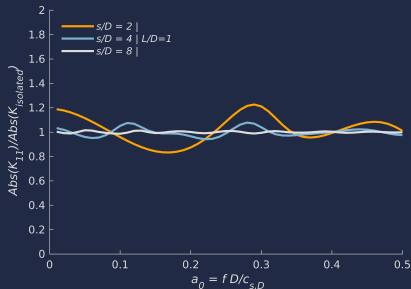


$$K_{11} = \frac{1}{4} (K_{ss} + 2K_{sa} + K_{aa})$$

$$K_{12} = \frac{1}{4} (K_{ss} - K_{aa})$$

$$K_{13} = \frac{1}{4} (K_{ss} - 2K_{sa} + K_{aa})$$

K_{11}, K_{12}, K_{13} vs s/D , $L/D = 1$, $J = 100$, $m = 0$



Outline

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 - ▶ Similar behavior between homogeneous and Gibson
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 - ▶ Similar behavior between homogeneous and Gibson
 - ▶ Criteria for neglecting interaction
- ▶ Future: closed-form formulae or simplified methodology for engineering purposes

Acknowledgments

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